

REF: NEX-2300100.00

December 10, 2025

Ms. Harmony Wilson, Chair
 Haverhill Conservation Commission
 City Hall Room 300
 4 Summer Street
 Haverhill, MA 01830

SUBJECT: Response to Comments – Notice of Intent Application
 Proposed Replacement of Brandy Brow Road Culvert over East Meadow River
 Haverhill, MA

Dear Ms. Wilson and Members of the Commission:

On Behalf of the City of Haverhill, Greenman-Pedersen, Inc. (GPI) has prepared the enclosed supplementary information and materials as a response to comments and questions provided by the Haverhill Conservation Commission and Staff at the public hearing held on November 19, 2025.

Modelling and analysis conducted to assess the impacts of the proposed culvert replacement indicate that the project will result in minor downstream increases in water surface elevations (WSEL) in the proposed condition for the 2-, 5-, 10-, 25-, & 50-year storm events, as demonstrated in **Table 1**, below.*

Table 1 – WSEL Comparison 50 feet and 200 feet Downstream of the Brandy Brow Culvert

Storm Frequency	Existing WSEL (ft)	Proposed WSEL (ft)	Δ (ft)
50 feet Downstream			
2-Year	62.30	62.30	0.00
5-Year	62.65	62.89	0.24
10-Year	62.86	63.63	0.77
25-Year	63.38	64.20	0.82
50-Year	64.23	64.55	0.32
100-Year	64.88	64.83	-0.05
200 feet Downstream			
2-Year	60.25	60.26	0.01
5-Year	60.61	60.85	0.24
10-Year	60.81	61.57	0.76
25-Year	61.32	62.09	0.77
50-Year	62.11	62.42	0.31
100-Year	62.72	62.68	-0.04

*This table is provided as **Table 4.2.d WSEL Comparison 50 feet and 200 feet Downstream of the Brandy Brow Culvert** on page 8 of the Hydraulic Report provided as Attachment C in the Notice of Intent Application.

Section 5.0 – Conclusions and Recommendations of the Hydraulic Report stated “However, the downstream water surface elevations increased for the 2-, 5-, 10-, 25- and 50-year storms due to the loss of storage caused by increasing the hydraulic capacity of the proposed culvert. The increases in water surface elevation should be investigated further to ensure offsite properties are not being affected by the

increases in water surface elevation for said storms due the culvert replacement” and “The proposed culvert can safely convey the 10-year design flood event without overtopping the roadway. The proposed structure will have a downstream impact on the stream hydraulics of East Meadow River due to the culvert replacement that should be investigated further to make sure no offsite properties are being flooded for the smaller storms discussed throughout this report.”

In order to ensure that the increases in WSEL during the 2-, 5-, 10-, 25-, & 50-year storm events would not negatively impact downstream properties and cause flooding, GPI plotted the existing and proposed WSEL on the plans and determined the lateral distance between the WSEL and downstream infrastructure. A supplementary plan set comparing the existing and proposed WSEL lateral distances is included as an enclosure. The results of this analysis are provided in **Table 2****, below.

Table 2 – Lateral Extent of Floodwater in 50-Year Storm Event

	Existing WSEL (ft)	Proposed WSEL (ft)	Δ (ft)
Lateral Distance to Structure at 284 Brandy Brow Road	100.5	98	-2.5
Lateral Distance to Driveway at 288 Brandy Brow Road	74.5	72	-2.5
Lateral Distance to Structure at 288 Brandy Brow Road	134	132.5	-1.5

****Table 2** provides analysis only for the 50-year storm event as it has the maximum anticipated lateral extent of floodwater. The replacement of the culvert will result in a net decrease in WSEL during the 100-year storm event.

The results of this analysis indicate that the anticipated maximum lateral extent of floodwaters during the 50-year storm event will extend between 1.5 and 2.5 further east / southeast towards the properties at 284 & 288 Brandy Brow Road, however, it is not anticipated that this increase will have any negative impact on either property. As previously stated, the proposed culvert replacement will reduce the WSEL during the 100-year storm event.

West of the culvert, the anticipated maximum lateral extent of floodwaters during the 50-year storm event upon replacement of the culvert will extend north and west and encompass the wooded area west of the culvert and south of Brandy Brow Road. It is not anticipated that this change will have any negative impacts as the existing area is forested and will function as a natural floodplain. Scour protection will be installed within the project limits to protect against erosion, scour, or undermining of the culvert.

South of the property at 288 Brandy Brow Road, the East Meadow River flows south through undeveloped forest and wetlands for approximately 1.5 miles before reaching the Amesbury Road (Route 110) crossing. Based on the available Federal Emergency Management Agency (FEMA) data, this area represents approximately 84.63 acres of floodplain. A USGS Topographic Locus Map detailing the downstream floodplain limits is provided as an enclosure. GPI anticipates that this area will provide adequate storage for any minor increase in WSEL and that no significant changes in WSEL or flow rate will be observed at the Route 110 crossing.

Per the Commission’s request, GPI has included a general note on Sheet 3 - Typical Sections, indicating, “THE EXISTING BRANDY BROW ROAD CULVERT SPANNING EAST MEADOW RIVER RESTRICTS ACCESS TO MOTOR VEHICLES. THE PROPOSED HMA MULTI-USE PEDESTRIAN PATH WILL MAINTAIN RESTRICTED ACCESS TO MOTOR VEHICLES. PROPOSED FOLD DOWN BOLLARDS

MAY BE DISENGAGED BY EMERGENCY PERSONNEL TO ALLOW TRAIL VEHICLE TRAVEL ACROSS THE CULVERT.” Additionally, revisions have been made to Sheet 9 – Key Plan, Locus Map, & Profiles. Text leaders of the profile sections were previously rotated and have been adjusted accordingly. These revisions are included in the enclosed NOI Plan Set.

The City of Haverhill and GPI hope that the Commission will find this analysis sufficient to address the comments provided to date. Should you have any additional questions, or require additional information, please contact me directly at (978) 570-2559.

Sincerely,

GREENMAN-PEDERSEN, INC.



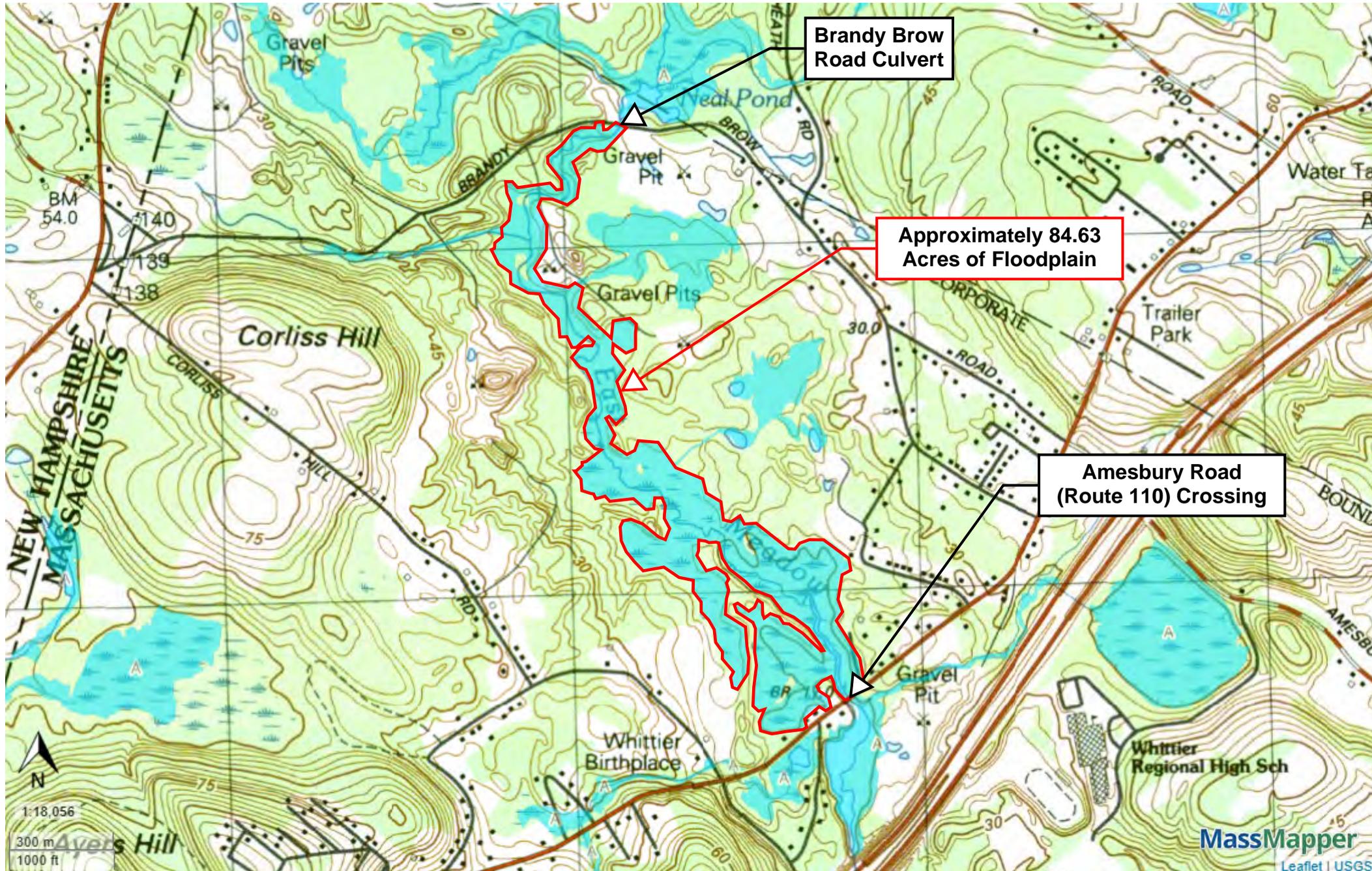
Alexa Marquis
Environmental Analyst

enclosure(s) USGS Topographic Locus Map of Downstream Floodplain
 WSEL Comparison Sheet
 WPA Notice of Intent Plans – Revised

- c. Billie Anne Stone, City of Haverhill – Engineering Department
Samuel Campbell, GPI – Environmental Department Head
Chris Stairs, GPI – Director of Transportation Engineering
Ryan Melchionno, GPI – Project Manager Structural Engineer
Sid Kashi, GPI – Senior Project Manager
Girish Patel, GPI – Senior Designer

USGS Topographic Locus Map of Downstream Floodplain

USGS TOPOGRAPHIC LOCUS MAP OF DOWNSTREAM FLOODPLAIN



FEMA National Flood Hazard Layer Polygons

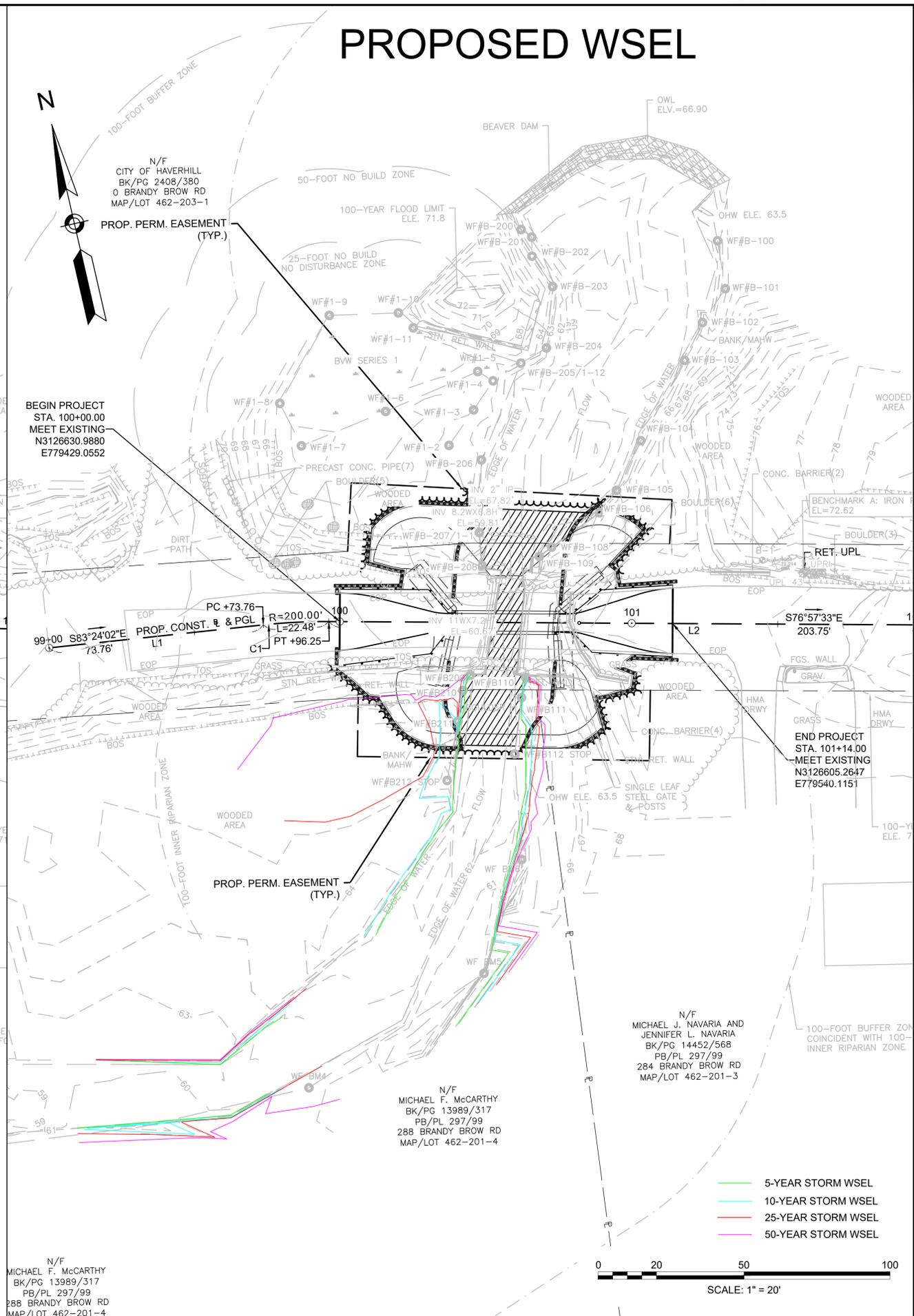
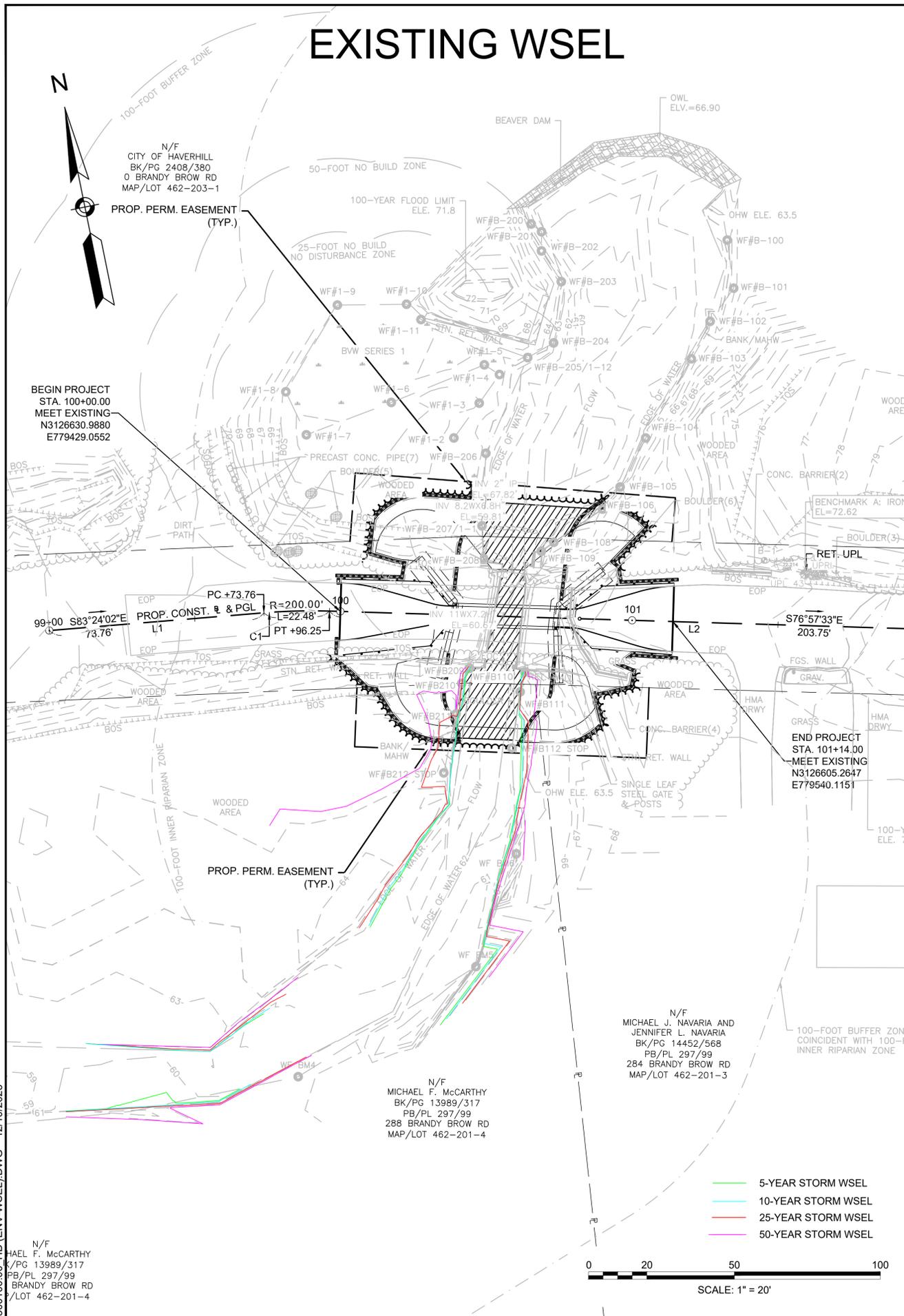
- 1% Annual Chance Flood Hazard
- Regulatory Floodway
- Area of Undetermined Flood Hazard
- 0.2% Annual Chance Flood Hazard
- Area with Reduced Risk Due to Levee
- Area Not Included

Replacement of Brandy Brow Road Culvert over East Meadow River
Haverhill, MA

WSEL Comparison Sheet

EXISTING WSEL

PROPOSED WSEL



PREPARED FOR
CITY OF HAVERHILL
4 SUMMER STREET
HAVERHILL, MA 01830

**REPLACEMENT OF BRANDY BROW ROAD
CULVERT OVER EAST MEADOW RIVER
BRANDY BROW ROAD
HAVERHILL, MASSACHUSETTS**

NO.	REVISION	DATE

REVISIONS		
NO.	REVISION	DATE

**WSEL
COMPARISON**

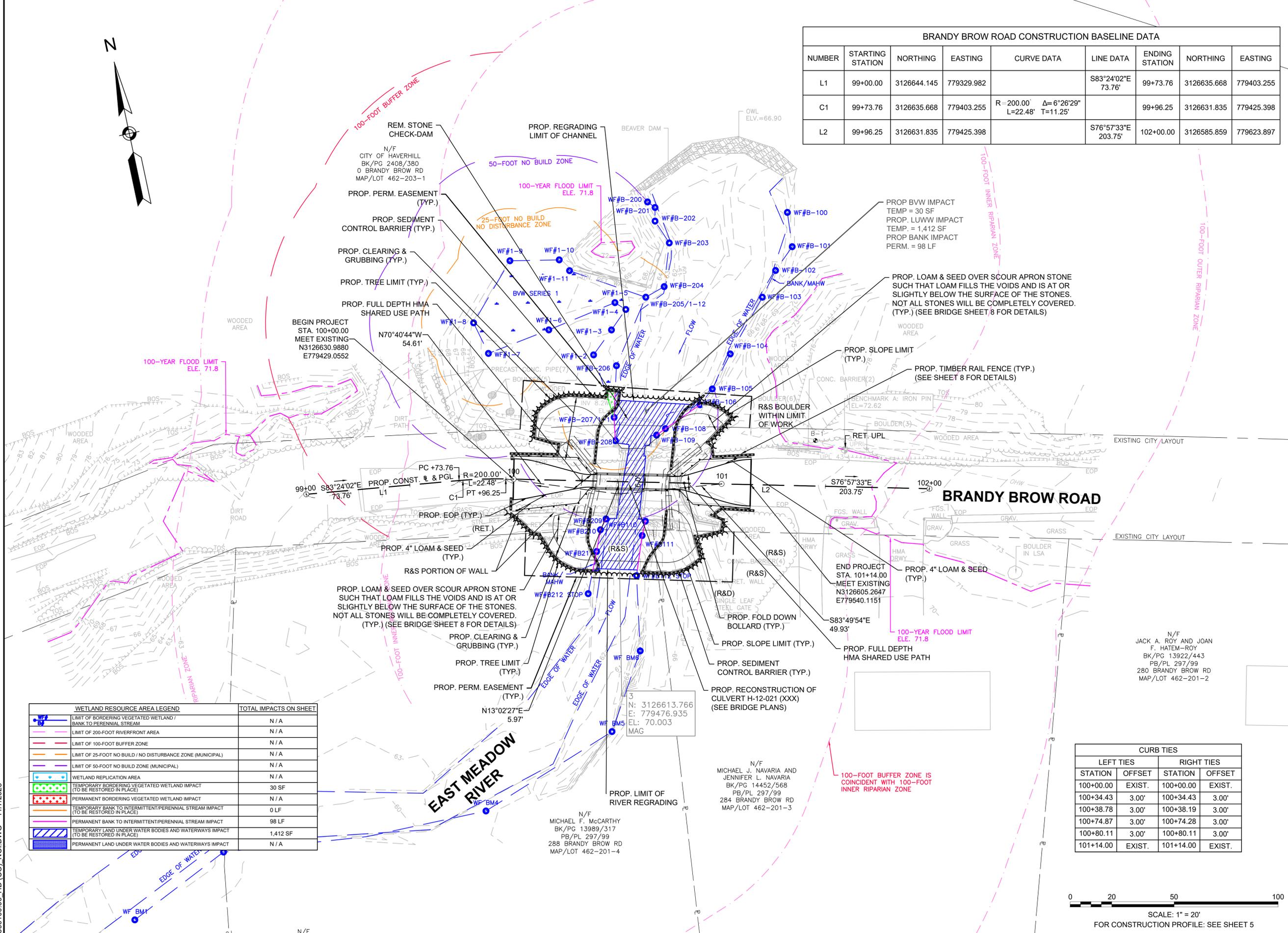
SCALE: 1"=20'
NEX-2300100.00
1 OF 1

2300100.00_HD (ENV-WSEL).DWG 12/10/2025

WPA Notice of Intent Plans – Revised

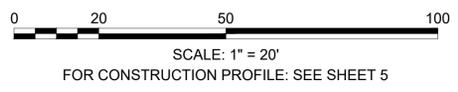
**REPLACEMENT OF BRANDY BROW ROAD
CULVERT OVER EAST MEADOW RIVER
BRANDY BROW ROAD
HAVERHILL, MASSACHUSETTS**

BRANDY BROW ROAD CONSTRUCTION BASELINE DATA								
NUMBER	STARTING STATION	NORTHING	EASTING	CURVE DATA	LINE DATA	ENDING STATION	NORTHING	EASTING
L1	99+00.00	3126644.145	779329.982		S83°24'02"E 73.76'	99+73.76	3126635.668	779403.255
C1	99+73.76	3126635.668	779403.255	R=200.00' Δ=6°26'29" L=22.48' T=11.25'		99+96.25	3126631.835	779425.398
L2	99+96.25	3126631.835	779425.398		S76°57'33"E 203.75'	102+00.00	3126585.859	779623.897



WETLAND RESOURCE AREA LEGEND		TOTAL IMPACTS ON SHEET
	LIMIT OF BORDERING VEGETATED WETLAND / BANK TO PERENNIAL STREAM	N/A
	LIMIT OF 200-FOOT RIVERFRONT AREA	N/A
	LIMIT OF 100-FOOT BUFFER ZONE	N/A
	LIMIT OF 25-FOOT NO BUILD / NO DISTURBANCE ZONE (MUNICIPAL)	N/A
	LIMIT OF 50-FOOT NO BUILD ZONE (MUNICIPAL)	N/A
	WETLAND REPLICATION AREA	N/A
	TEMPORARY BORDERING VEGETATED WETLAND IMPACT (TO BE RESTORED IN PLACE)	30 SF
	PERMANENT BORDERING VEGETATED WETLAND IMPACT	N/A
	TEMPORARY BANK TO INTERMITTENT/PERENNIAL STREAM IMPACT (TO BE RESTORED IN PLACE)	0 LF
	PERMANENT BANK TO INTERMITTENT/PERENNIAL STREAM IMPACT	98 LF
	TEMPORARY LAND UNDER WATER BODIES AND WATERWAYS IMPACT (TO BE RESTORED IN PLACE)	1,412 SF
	PERMANENT LAND UNDER WATER BODIES AND WATERWAYS IMPACT	N/A

CURB TIES			
LEFT TIES		RIGHT TIES	
STATION	OFFSET	STATION	OFFSET
100+00.00	EXIST.	100+00.00	EXIST.
100+34.43	3.00'	100+34.43	3.00'
100+38.78	3.00'	100+38.19	3.00'
100+74.87	3.00'	100+74.28	3.00'
100+80.11	3.00'	100+80.11	3.00'
101+14.00	EXIST.	101+14.00	EXIST.



REVISIONS

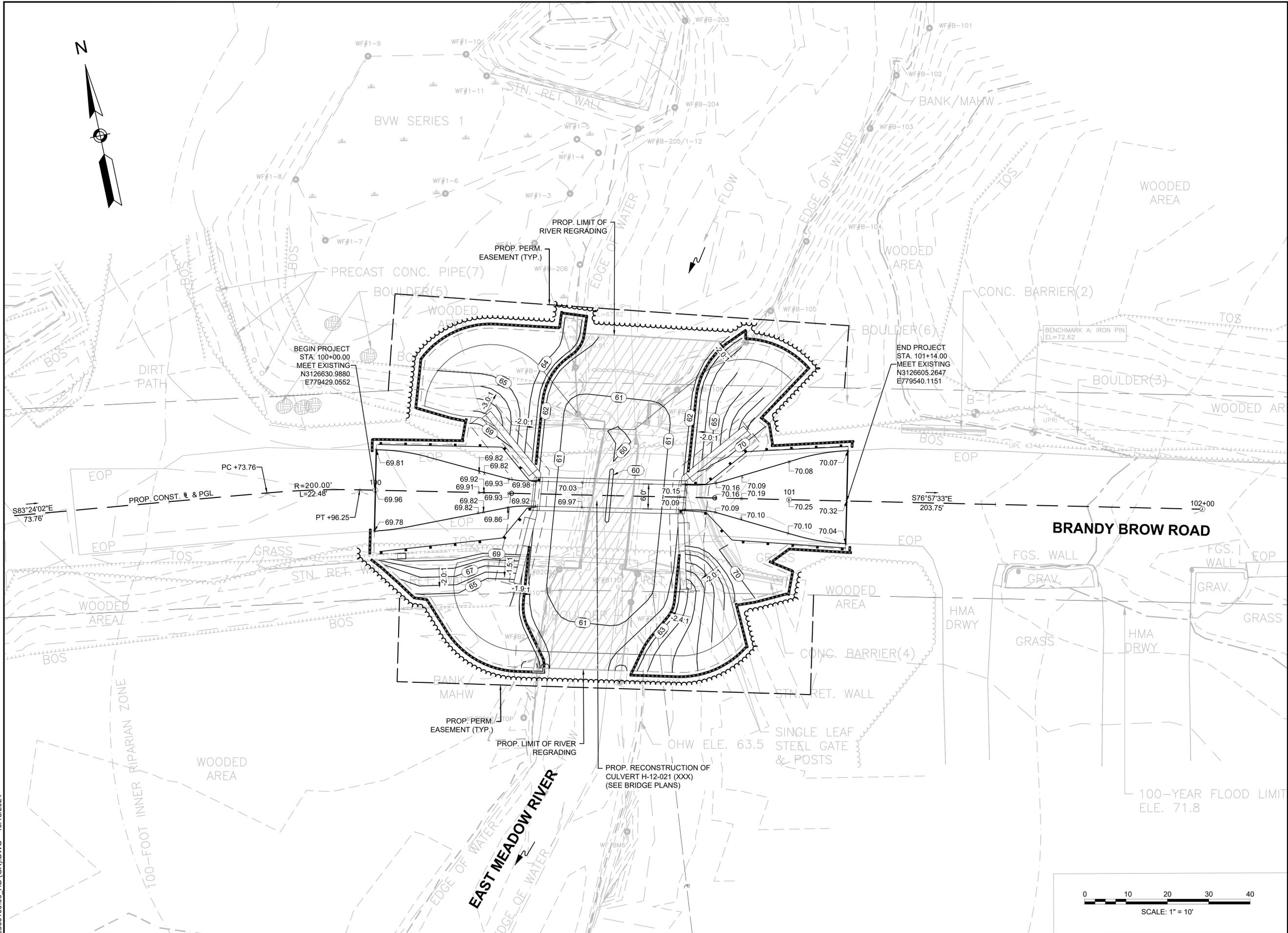
NO.	REVISION	DATE

11/6/2025
DRAWN/DESIGN BY: NHG/GBP
CHECKED BY: GJH

CONSTRUCTION PLAN

SCALE: 1"=20'
NEX-2300100.00
4 OF 20

**REPLACEMENT OF BRANDY BROW ROAD
CULVERT OVER EAST MEADOW RIVER
BRANDY BROW ROAD
HAVERHILL, MASSACHUSETTS**



REVISIONS		
NO.	REVISION	DATE

REVISIONS		
NO.	REVISION	DATE

GRADING PLAN

SCALE: 1"=10'

NEX-2300100.00

6 OF 20

2300100.00_HD (GR).DWG 10/15/2024

BORING B-1

BORING INFORMATION		BORING																																																																																										
LOCATION: See Site Plan		B-1 PAGE 1 of 2																																																																																										
GROUND SURFACE EL. (ft): -70	DATE START/END: 1/23/2024 - 1/23/2024																																																																																											
VERTICAL DATUM: Project Datum	DRILLING COMPANY: Northern Drill Service, Inc.																																																																																											
TOTAL DEPTH (ft): 51.0	DRILLER NAME: T. Tucker																																																																																											
LOGGED BY: A. Han	RIG TYPE: Mobile B-57																																																																																											
DRILLING INFORMATION																																																																																												
HAMMER TYPE: Automatic	CASING I.D./O.D.: 4 inch/4.25 inch	CORE BARREL TYPE:																																																																																										
AUGER I.D./O.D.: NA / NA	DRILL ROD O.D.: NM	CORE BARREL I.D./O.D. NA / NA																																																																																										
DRILLING METHOD: Drive and Wash																																																																																												
WATER LEVEL DEPTHS (ft): ∇ 8.6 1/23/2024 11:45 am																																																																																												
ABBREVIATIONS:																																																																																												
Pen. = Penetration Length	S = Split Spoon Sample	Qp = Pocket Penetrometer Strength	NA, NM = Not Applicable, Not Measured																																																																																									
Rec. = Recovery Length	C = Core Sample	Sv = Pocket Torvane Shear Strength	Blows per 6 in.: 140-lb hammer falling																																																																																									
RQD = Rock Quality Designation	U = Undisturbed Sample	LL = Liquid Limit	30 inches to drive a 2-inch-O.D. split spoon sampler.																																																																																									
= Length of Sound Cores*4 in / Pen. %	SC = Sonic Core	PI = Plasticity Index																																																																																										
WOR = Weight of Rods	DP = Direct Push Sample	PID = Photoionization Detector																																																																																										
WOH = Weight of Hammer	HSA = Hollow-Stem Auger	I.D./O.D. = Inside Diameter/Outside Diameter																																																																																										
<table border="1"> <thead> <tr> <th>Elev. (ft)</th> <th>Depth (ft)</th> <th>Sample No.</th> <th>Depth (ft)</th> <th>Pen./ Rec. (in)</th> <th>Blows per 6 in. or RQD</th> <th>Drilling Remarks/ Field Test Data</th> <th>Layer Name</th> <th>Soil and Rock Description</th> </tr> </thead> <tbody> <tr> <td>70</td> <td></td> <td>S1</td> <td>0 to 2</td> <td>24/20</td> <td>60-73-26-27</td> <td rowspan="2">Drove casing and rollerbit to 4'.</td> <td>GRAVEL</td> <td>S1 (0"-4"): Asphalt. S1 (4"-20"): Moist, very dense, brown, FINE TO COARSE GRAVEL, some fine to coarse sand, trace inorganic silt.</td> </tr> <tr> <td></td> <td></td> <td>S2</td> <td>2 to 4</td> <td>24/12</td> <td>20-19-14-11</td> <td>S2: Moist, dense, brown, FINE TO COARSE SAND, some fine to coarse gravel, trace inorganic silt.</td> </tr> <tr> <td>65</td> <td>5</td> <td>S3</td> <td>4 to 6</td> <td>24/10</td> <td>8-7-6-4</td> <td rowspan="2">Drove casing and rollerbit to 10'.</td> <td>SAND</td> <td>S3: Moist, medium dense, orange, FINE TO COARSE SAND, some fine to coarse gravel, trace inorganic silt.</td> </tr> <tr> <td></td> <td></td> <td>S4</td> <td>6 to 8</td> <td>24/11</td> <td>4-3-2-5</td> <td>S4: Moist, loose, orange, FINE TO COARSE SAND, some fine to coarse gravel, trace inorganic silt.</td> </tr> <tr> <td>60</td> <td>10</td> <td>S5</td> <td>8 to 10</td> <td>24/9</td> <td>6-8-10-8</td> <td rowspan="2">Drove casing and rollerbit to 14'.</td> <td>GRAVEL</td> <td>S5: Wet, medium dense, orange, FINE TO COARSE GRAVEL, some fine to coarse sand.</td> </tr> <tr> <td></td> <td></td> <td>S6</td> <td>10 to 12</td> <td>24/9</td> <td>6-9-12-12</td> <td>S6: Wet, medium dense, brown, FINE TO COARSE GRAVEL, some fine to coarse sand, trace inorganic silt.</td> </tr> <tr> <td></td> <td></td> <td>S7</td> <td>12 to 14</td> <td>24/13</td> <td>16-7-9-9</td> <td rowspan="2">Drove casing and rollerbit to 19'.</td> <td>GRAVEL</td> <td>S7: Moist, medium dense, brown, FINE TO COARSE GRAVEL, some fine to coarse sand, trace inorganic silt.</td> </tr> <tr> <td></td> <td></td> <td>S8</td> <td>14 to 16</td> <td>24/9</td> <td>7-12-9-11</td> <td>S8: Wet, medium dense, brown, FINE TO COARSE GRAVEL, some fine to coarse sand.</td> </tr> <tr> <td>55</td> <td>15</td> <td></td> <td></td> <td></td> <td></td> <td rowspan="2">Drove casing and rollerbit to 19'.</td> <td>SAND</td> <td>S9: Wet, loose, grey, FINE TO COARSE SAND.</td> </tr> <tr> <td></td> <td></td> <td>S9</td> <td>19 to 21</td> <td>24/4</td> <td>4-2-2-2</td> <td></td> </tr> </tbody> </table>				Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and Rock Description	70		S1	0 to 2	24/20	60-73-26-27	Drove casing and rollerbit to 4'.	GRAVEL	S1 (0"-4"): Asphalt. S1 (4"-20"): Moist, very dense, brown, FINE TO COARSE GRAVEL, some fine to coarse sand, trace inorganic silt.			S2	2 to 4	24/12	20-19-14-11	S2: Moist, dense, brown, FINE TO COARSE SAND, some fine to coarse gravel, trace inorganic silt.	65	5	S3	4 to 6	24/10	8-7-6-4	Drove casing and rollerbit to 10'.	SAND	S3: Moist, medium dense, orange, FINE TO COARSE SAND, some fine to coarse gravel, trace inorganic silt.			S4	6 to 8	24/11	4-3-2-5	S4: Moist, loose, orange, FINE TO COARSE SAND, some fine to coarse gravel, trace inorganic silt.	60	10	S5	8 to 10	24/9	6-8-10-8	Drove casing and rollerbit to 14'.	GRAVEL	S5: Wet, medium dense, orange, FINE TO COARSE GRAVEL, some fine to coarse sand.			S6	10 to 12	24/9	6-9-12-12	S6: Wet, medium dense, brown, FINE TO COARSE GRAVEL, some fine to coarse sand, trace inorganic silt.			S7	12 to 14	24/13	16-7-9-9	Drove casing and rollerbit to 19'.	GRAVEL	S7: Moist, medium dense, brown, FINE TO COARSE GRAVEL, some fine to coarse sand, trace inorganic silt.			S8	14 to 16	24/9	7-12-9-11	S8: Wet, medium dense, brown, FINE TO COARSE GRAVEL, some fine to coarse sand.	55	15					Drove casing and rollerbit to 19'.	SAND	S9: Wet, loose, grey, FINE TO COARSE SAND.			S9	19 to 21	24/4	4-2-2-2	
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NOTES: Driller notes that he maintained a head on the casing as rods were pulled during sampling.		PROJECT NAME: Brandy Brow Road Over East Meadow River CITY/STATE: Haverhill, Massachusetts GEI PROJECT NUMBER: 2400197																																																																																										

BORING NOTES:

- LOCATION OF BORINGS SHOWN ON THE PLAN THUS: \odot B-1
- BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- WATER LEVELS SHOWN ON THE BORING LOGS WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL.
- FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 1 3/8" I.D. SPLIT SPOON SAMPLER 6" USING A 140 POUND WEIGHT FALLING 30".
- CONTRACTOR MAY EXAMINE THE SOIL AND ROCK SAMPLES BY CONTACTING GREENMAN-PEDERSEN, INC.
- BORING B1 WAS MADE ON JANUARY 23, 2024.
- BORINGS WERE MADE BY NORTHERN DRILL SERVICE, INC. OF NORTHBOROUGH, MA UNDER THE GUIDANCE OF GEI CONSULTANTS, INC.
- THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.
- GROUND SURFACE ELEVATIONS ARE APPROXIMATE.

BORING LOG
SCALE: 3/8" = 1'-0"



BORING B-1 (CONT.)

BORING INFORMATION		BORING																																																																
LOCATION: See Site Plan		B-1 PAGE 2 of 2																																																																
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NOTES: Driller notes that he maintained a head on the casing as rods were pulled during sampling.		PROJECT NAME: Brandy Brow Road Over East Meadow River CITY/STATE: Haverhill, Massachusetts GEI PROJECT NUMBER: 2400197																																																																

PREPARED FOR
CITY OF HAVERHILL
4 SUMMER STREET
HAVERHILL, MA 01830

REPLACEMENT OF BRANDY BROW ROAD
 CULVERT OVER EAST MEADOW RIVER
 BRANDY BROW ROAD
 HAVERHILL, MASSACHUSETTS

REVISIONS

NO.	REVISION	DATE

11/6/2025

DRAWN/DESIGN BY CMP	CHECKED BY RWM
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BORING LOG

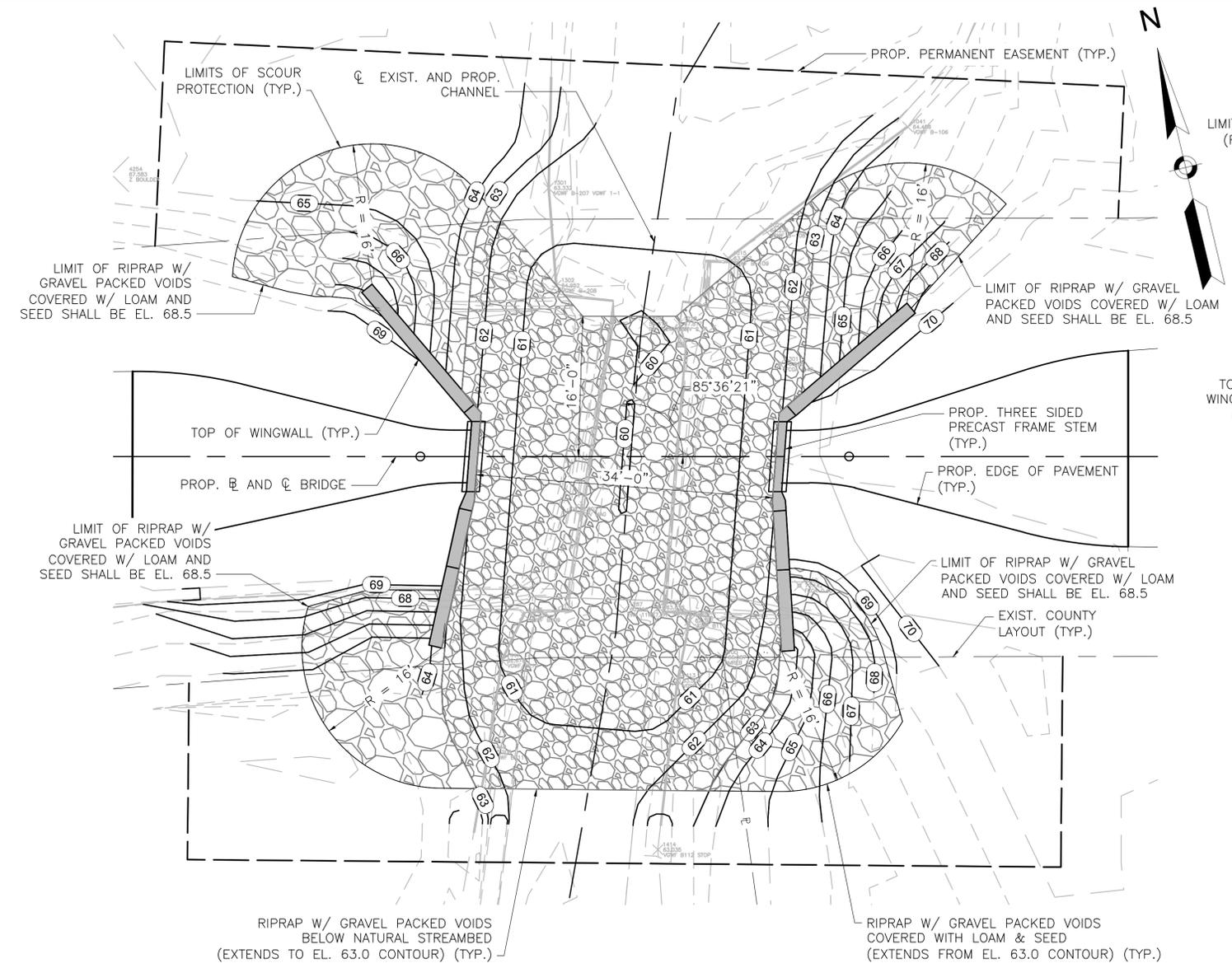
SCALE: AS NOTED

NEX2300100.00

11 OF 20

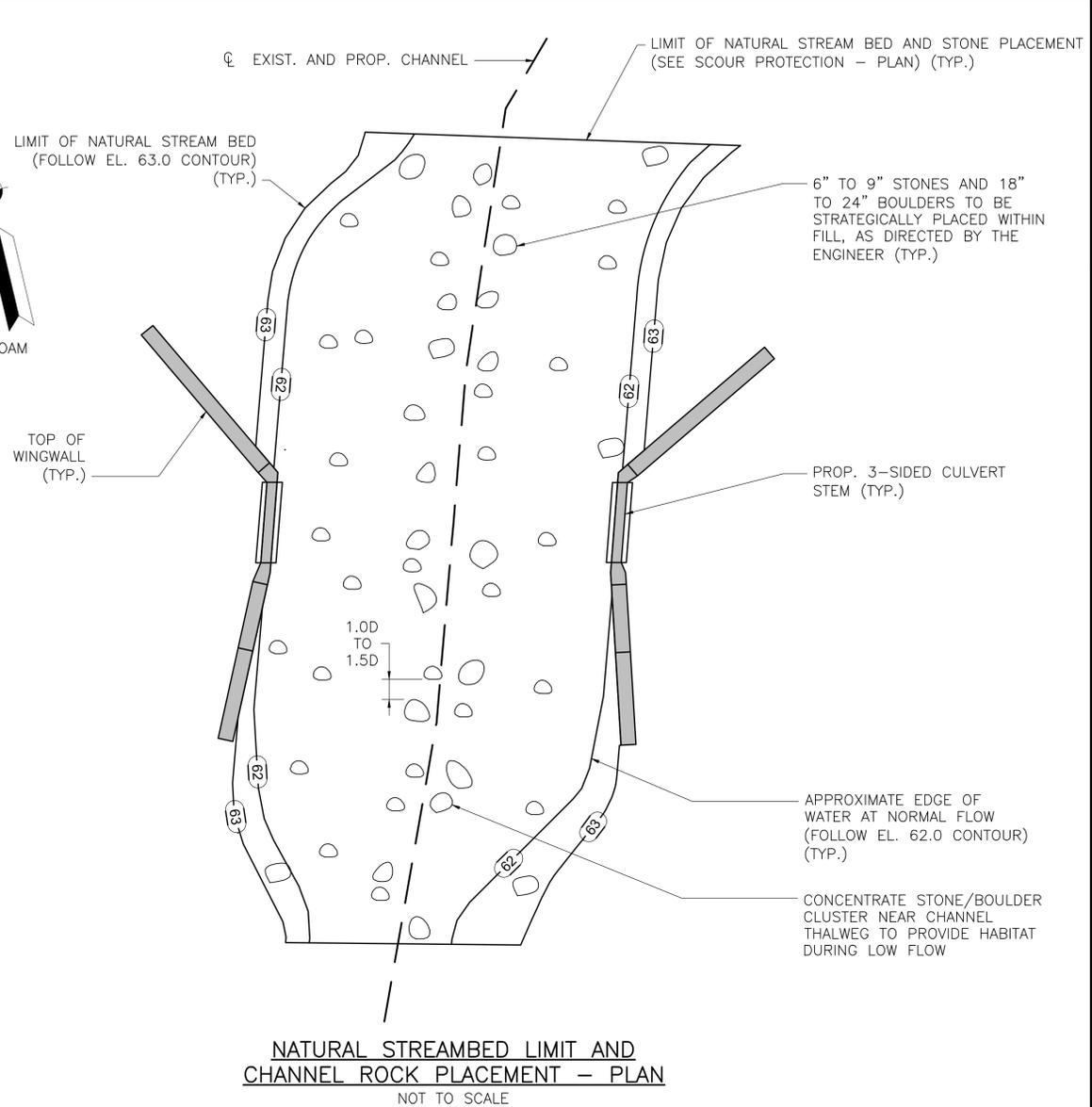
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**REPLACEMENT OF BRANDY BROW ROAD
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 BRANDY BROW ROAD
 HAVERHILL, MASSACHUSETTS**

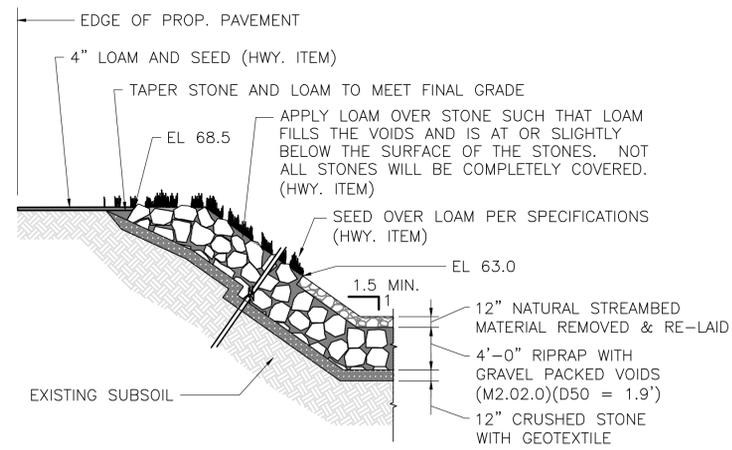


SCOUR PROTECTION - PLAN
 SCALE: 1/8" = 1'-0"

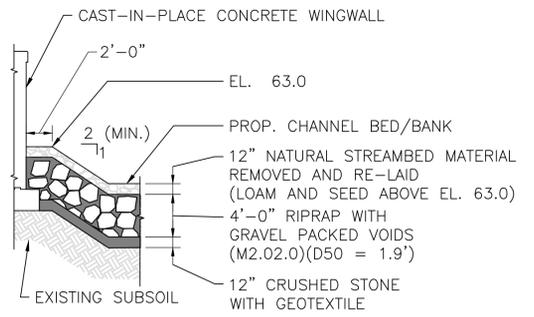
- KEY**
- RIPRAP W/GRAVEL PACKED VOIDS BELOW NATURAL STREAMBED
 - RIPRAP W/GRAVEL PACKED VOIDS COVERED W/LOAM AND SEED



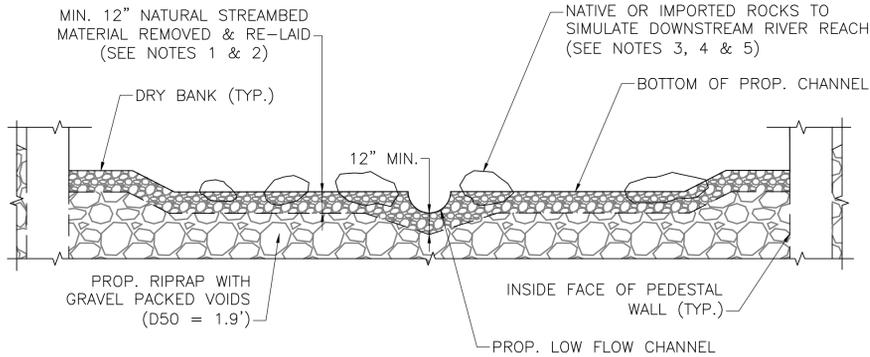
NATURAL STREAMBED LIMIT AND CHANNEL ROCK PLACEMENT - PLAN
 NOT TO SCALE



LOAM & SEED OVER RIPRAP DETAIL (NON-WATERWAY)
 NOT TO SCALE



RIPRAP AT WALL DETAIL (WATERWAY)
 NOT TO SCALE



CHANNEL ROCK PLACEMENT DETAIL
 NOT TO SCALE

RECONSTRUCTED CHANNEL NOTES:

1. NATURAL STREAMBED MATERIAL SHALL BE PLACED IN MAXIMUM 4" LIFTS. THE CONTRACTOR SHALL WASH FINES INTO THE LIFTS TO FILL VOIDS TO THE SATISFACTION OF THE ENGINEER TO PREVENT SUBSURFACE WATER MIGRATION.
2. NATURAL AND/OR IMPORTED STREAMBED MATERIAL SHALL MATCH EXISTING STREAM BED CHARACTERISTICS/GRADATION IN THE UNDISTURBED REACH GREATER THAN 175 LF DOWNSTREAM OF THE CULVERT.
3. EXCAVATED STONES (COBBLES AND BOULDERS) SHALL BE SALVAGED AND STOCKPILED ON SITE. THESE SALVAGED MATERIALS SHALL BE RE-USED IN THE RECONSTRUCTED CHANNEL TO SIMULATE THE NATURAL DOWNSTREAM REACH CONDITIONS.
4. EXCAVATED STONE MATERIAL THAT IS MAN-MADE OR NOT MATCHING THE DOWNSTREAM REACH CONDITIONS SHALL BE REMOVED AND DISCARDED.
5. BOULDER PLACEMENT AND EMBEDMENT IS SCHEMATICALLY SHOWN AND SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER BASED ON EXISTING FIELD CONDITIONS.
6. ALL COSTS FOR PLACING NATIVE/IMPORTED ROCKS IN THE PROPOSED CHANNEL SHALL BE INCLUDED IN ITEM 983.35, STREAMBED MATERIAL REMOVED AND REPLAID.

REVISIONS		
NO.	REVISION	DATE

SCOUR PROTECTION PLAN & CHANNEL DETAILS

SCALE: AS NOTED
 NEX2300100.00

BR09-RIPRAP DETAILS.DWG 11/7/2025

